



NHERI TallWood Project: Seismically Resilient Tall Wood Buildings

Co-PI and Collaborator in NHERI TallWood:

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Steve Pryor	Simpson Strong-Tie
John W. van de Lindt	Colorado State Univ.
Jeffrey Berman	University of Washington
Tara Hutchinson	Univ. of California San Diego
Reid Zimmerman	KPFF Consulting Engineers
James D Dolan	Washington State Univ.

Shiling Pei, Ph.D. P.E.
Colorado School of Mines

My Background

Shiling Pei, Ph.D, PE, F. SEI & ASCE
Associate Prof. Colorado School of Mines
Email: spei@mines.edu

Twitter: [@slpei](https://twitter.com/slpei) Website: <http://nheritallwood.mines.edu/>

- Conducting wood building research since 2004
- Start doing MT research in 2010
- A two-story test in 2017
- A 10-story test in 2023
- Working on P695 study on Mass Timber RW



PhD 2007, Colorado State University
Advisor: John W van de Lindt



2009 NEESWood Project
(post-doc work)



2017 2-story test



2023 NHERI TallWood Project

(Lead PI on NHERI TallWood Project)

What is a **Seismically Resilient Building**?

2011 Christchurch Earthquake M6.2





Credit: Elle Hunt <https://www.theguardian.com/>



Credit: Elle Hunt <https://www.theguardian.com/>



Credit: Elle Hunt <https://www.theguardian.com/>



Credit: Elle Hunt <https://www.theguardian.com/>

This is what you get by following
typical modern building codes
(maybe except in Japan)

**Total Buildings Demolished
AFTER the earthquake**

1200+

Total Casualty

185

Can Wood Building do better?

**Total Buildings Demolished
AFTER the earthquake**

0

Total Casualty

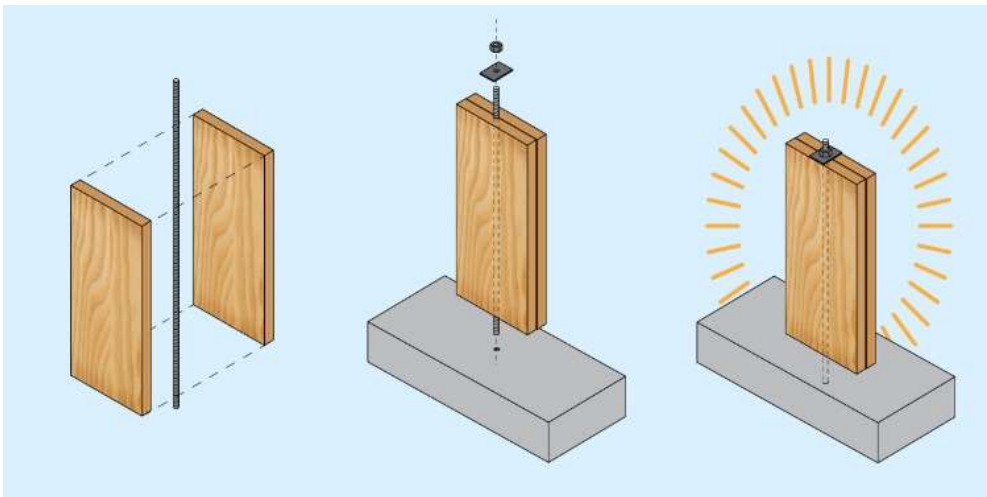
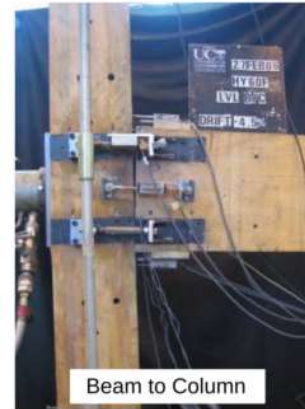
0

Could we do this?

In Theory This is Doable



WOOD
Push Puppet
on Steroid



Post-tensioned Mass Timber Rocking Wall



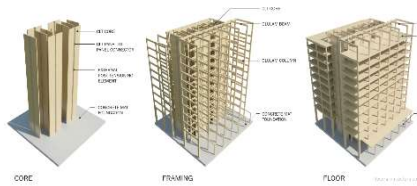
NHERI TallWood Project

Project duration: 2016~2023

Developing Resilience-based Seismic Design Method for Tall Wood buildings



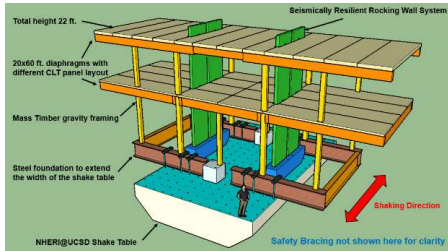
Concept: What can sell?



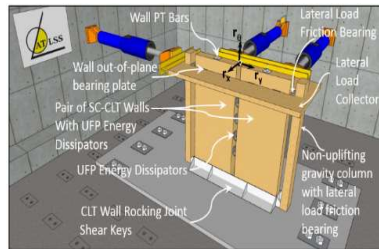
Define Tall Wood Archetypes



Small scale trials: Investigative testing



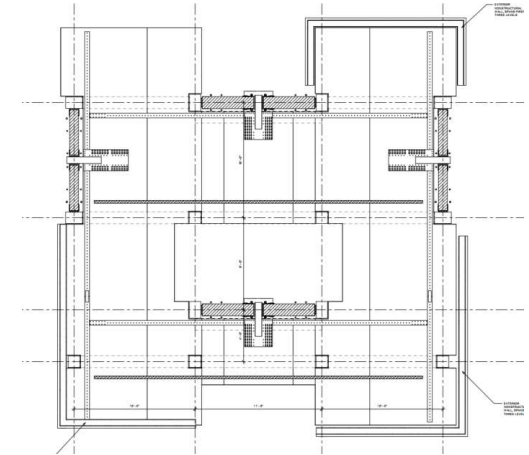
Two-story test at
NHERI@UCSD
2017 Summer



Assembly test at
NHERI@Lehigh
2019-2020



Validation: Full-scale 10-story Test (2023)

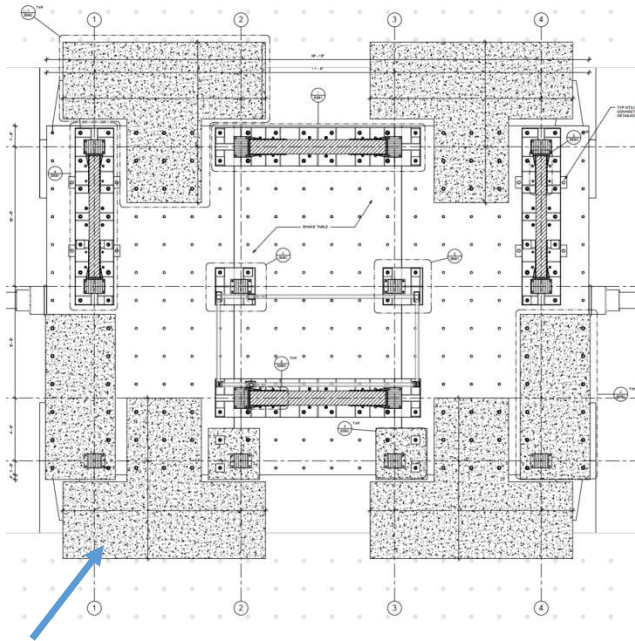


Seismic R & D
(2018~2022)

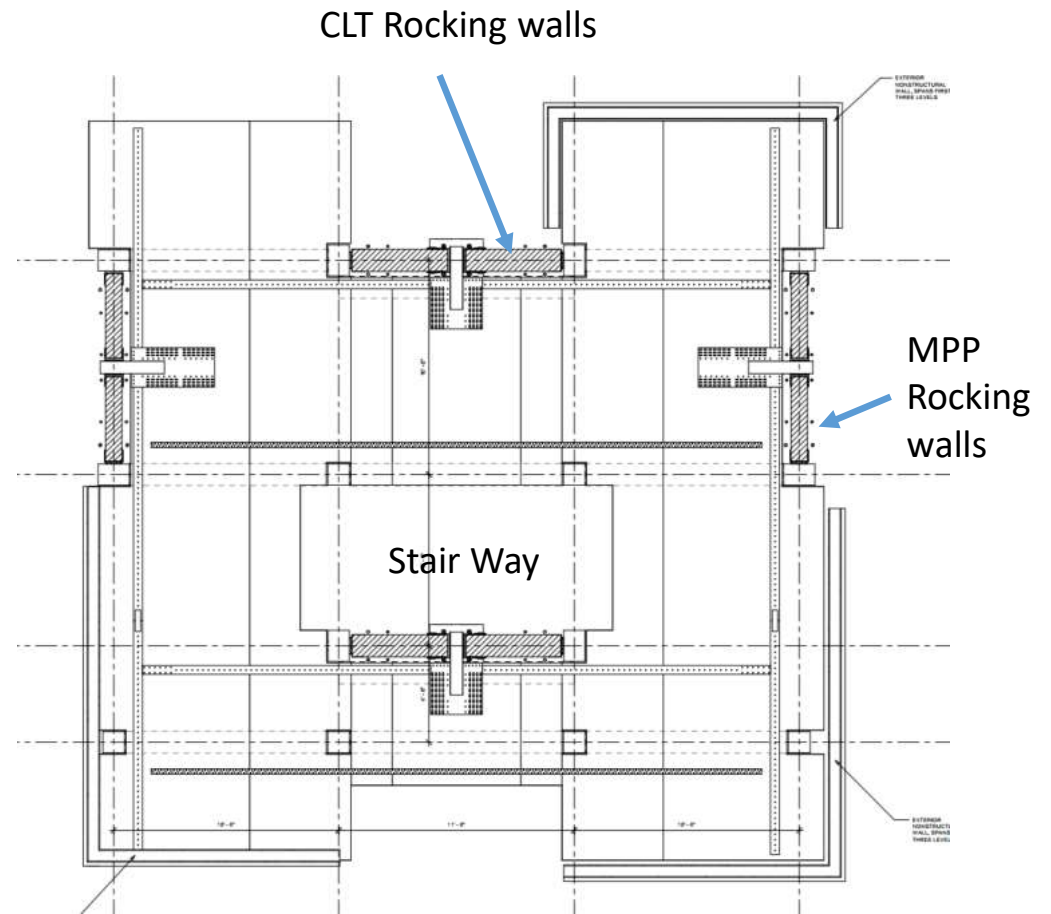


Building Design

Shake table size 40 x 25 ft (12.2 x 7.6 m)



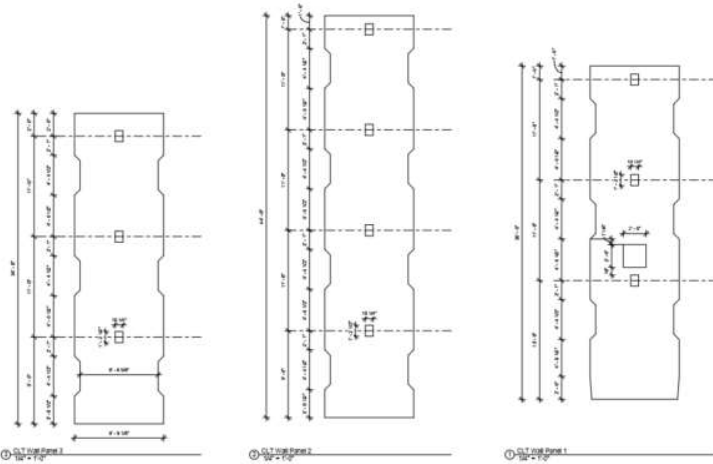
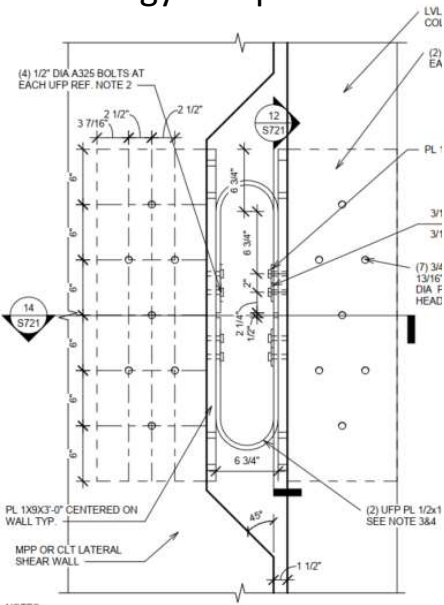
Use concrete overhang foundation to expand Shake table base



A floor plan of 35 x 32 ft (10.6 x 9.8 m)

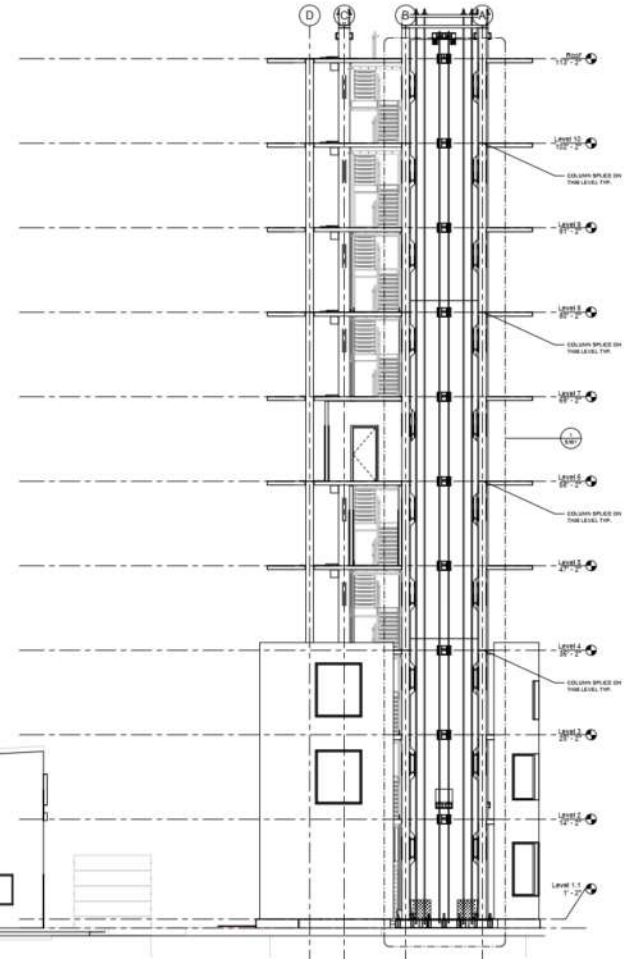
Elevation & Rocking Wall

Using UFP (U-shaped flexural plate) for Energy dissipation



Rocking wall panels assembled with 3 segments

Total height 110 ft (~33m)



Busch, A., R. B. Zimmerman, S. Pei, E. McDonnell, P. Line, and D. Huang. "Prescriptive seismic design procedure for post-tensioned mass timber rocking walls." *Journal of Structural Engineering* 148, no. 3 (2022): 04021289.



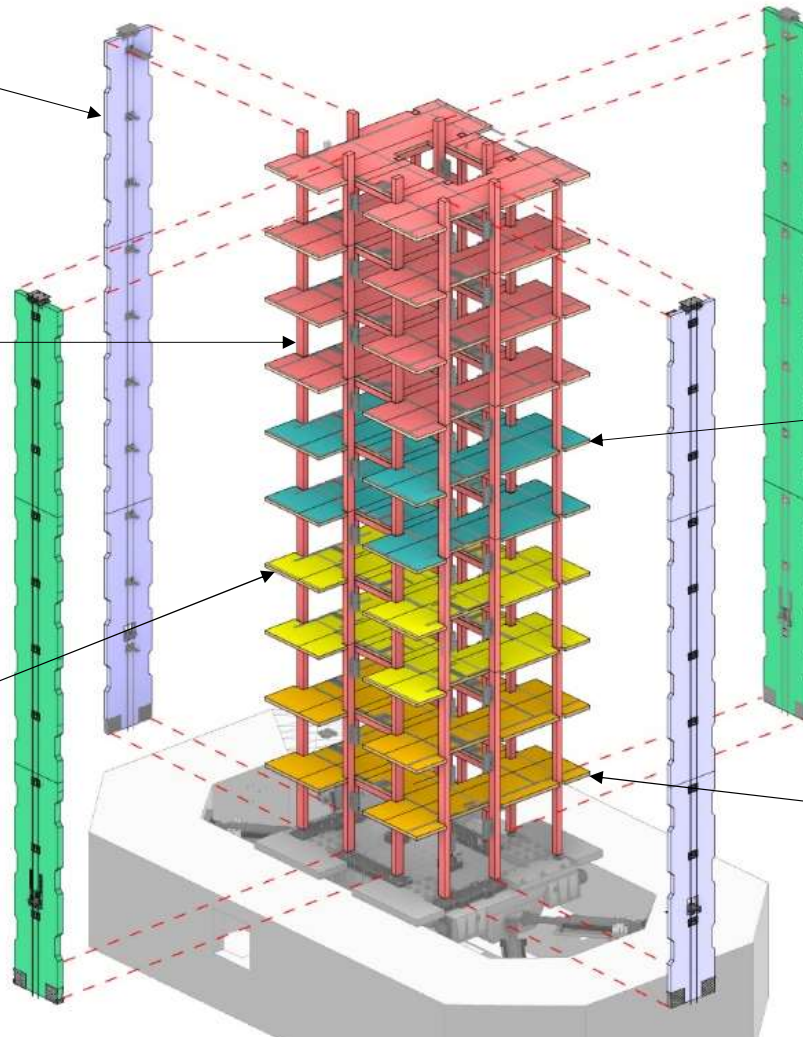
N-S Rocking Walls
Mass Plywood Panel
Spruce Pine Fir



Floors 7-10 and Columns and Beams
Veneer Laminated Timber
Douglas Fir



Floors 3-4
Glue Laminated Timber
European Spruce



E-W Rocking Walls
Cross Laminated Timber
Southern Pine



Floors 5-6
Nail/Dowel Laminated Timber
Spruce Pine Fir



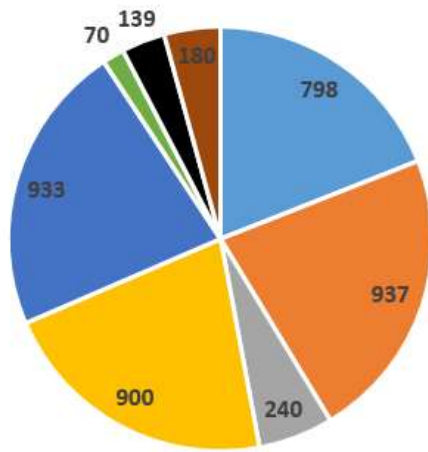
Floors 1-2
Cross Laminated Timber
European Spruce

NHERI TallWood

<http://nheritallwood.mines.edu/>

Now you have a design: **Fund-raise to build it**

10-Story Tallwood Test Total Cost
\$ 4.20 M



- Mat-Wood
- Mat-Steel
- Mat-NS
- Engineering
- Construction
- Demolishing
- UCSD charge/Test cost
- Test Personnel and Lodging

University



Industry partner



Construction/Design partner



Non-Prof Sponsor



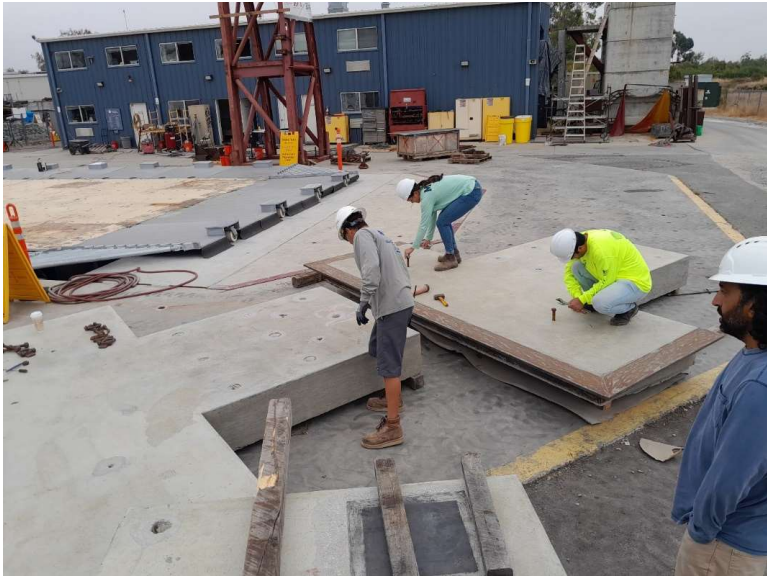
Construction: First thing first, foundations



Foundation needed for expanding shake table

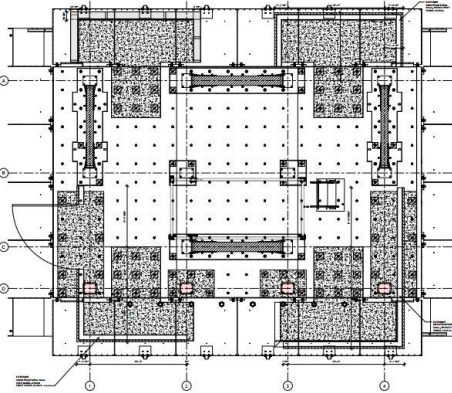
This was done by the **end of May 2022**





Early June

Placing and Post-tensioning of the foundation blocks and beams



July

Swinerton arrived on site after 7/4
Stair first, then Column of the first 3 floors
CLT floors.



Rocking Wall Panels

Each Rocking wall consists of 3 segments

Manufacture



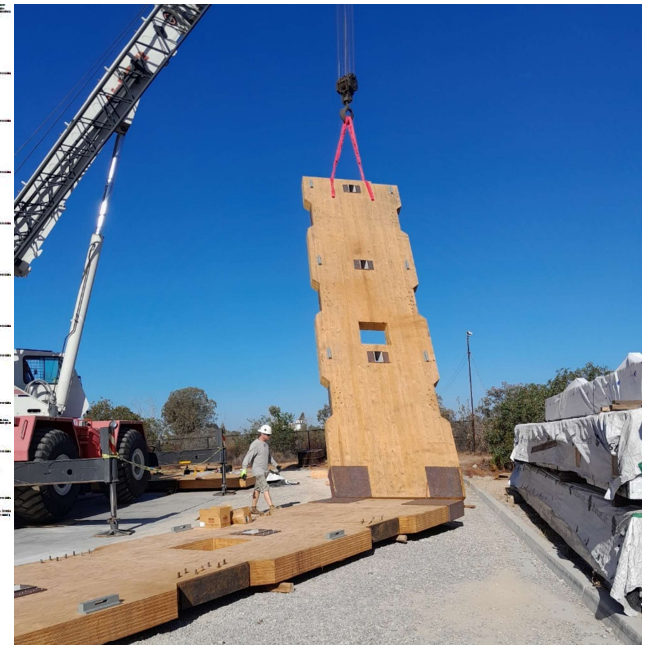
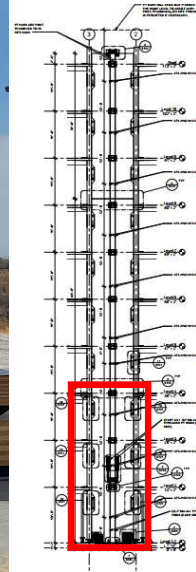
Fabricate and ship



Arrive on site



Hardware prep and install

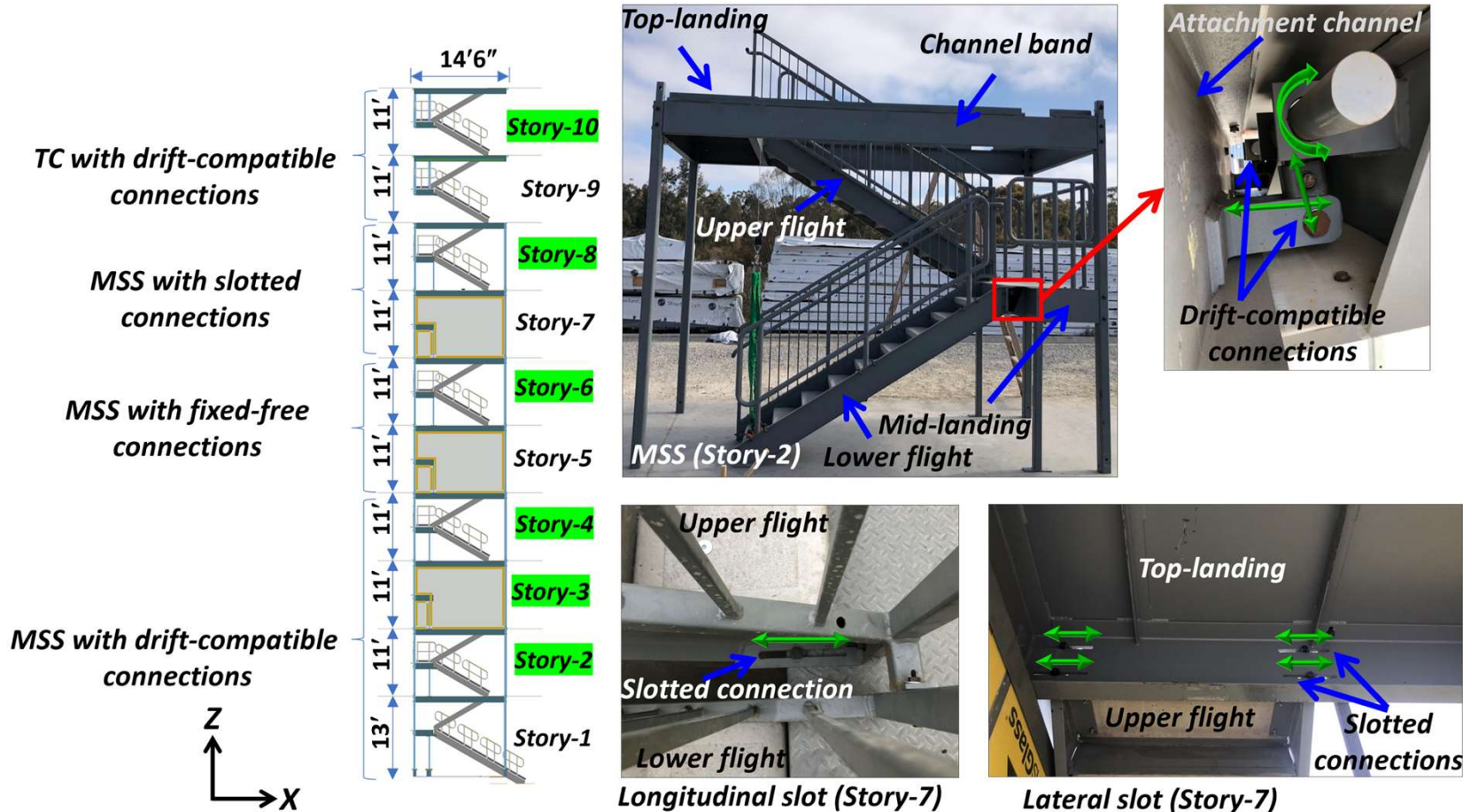






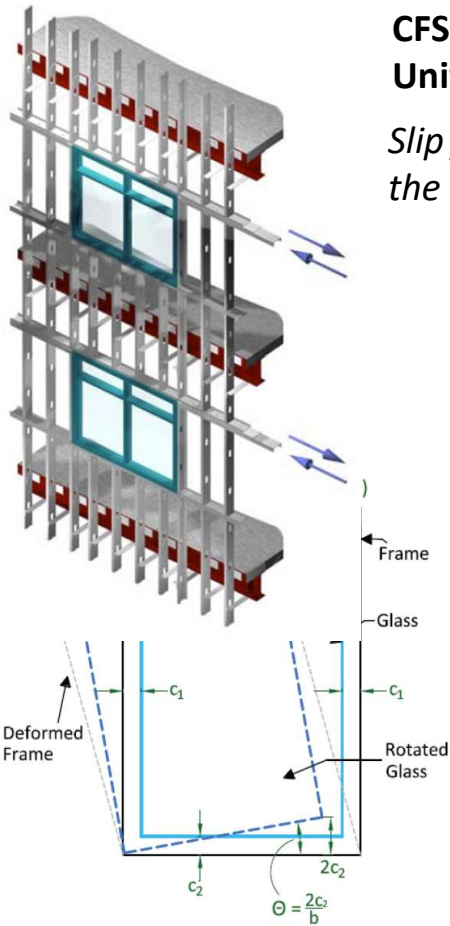


Features of Stair Testing



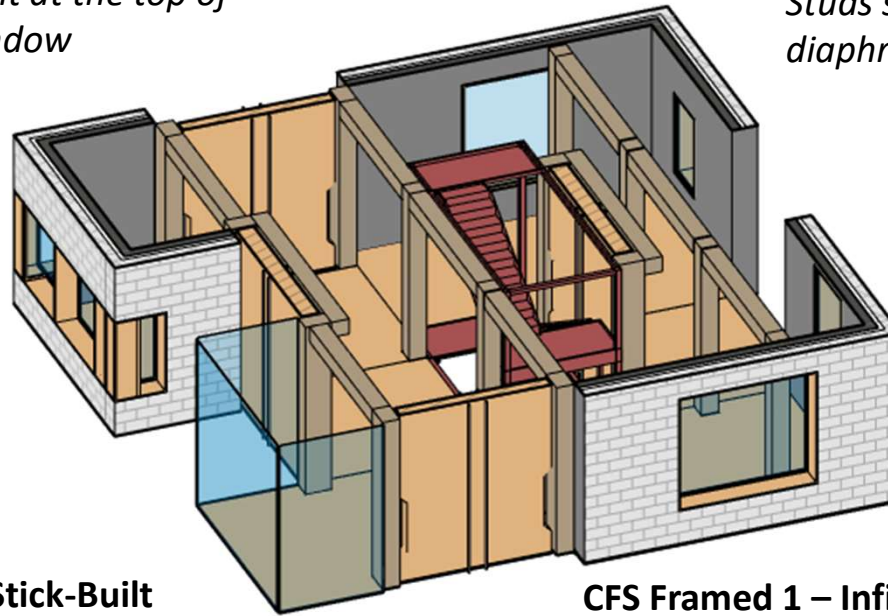
Credit: Prof. Tara Hutchinson, UCSD

Basic Exterior Wall Configurations



CFS Framed 3 – Spandrel Units with Ribbon Windows

Slip joint at the top of the window



Stick-Built Glass Curtain Wall

Glass rotates within the frame

CFS Framed 2 – Bypass Framed with Drift Clips

Studs slip relative to diaphragm



CFS Framed 1 – Infill or Platform Framed

Slip joint at the top of the wall

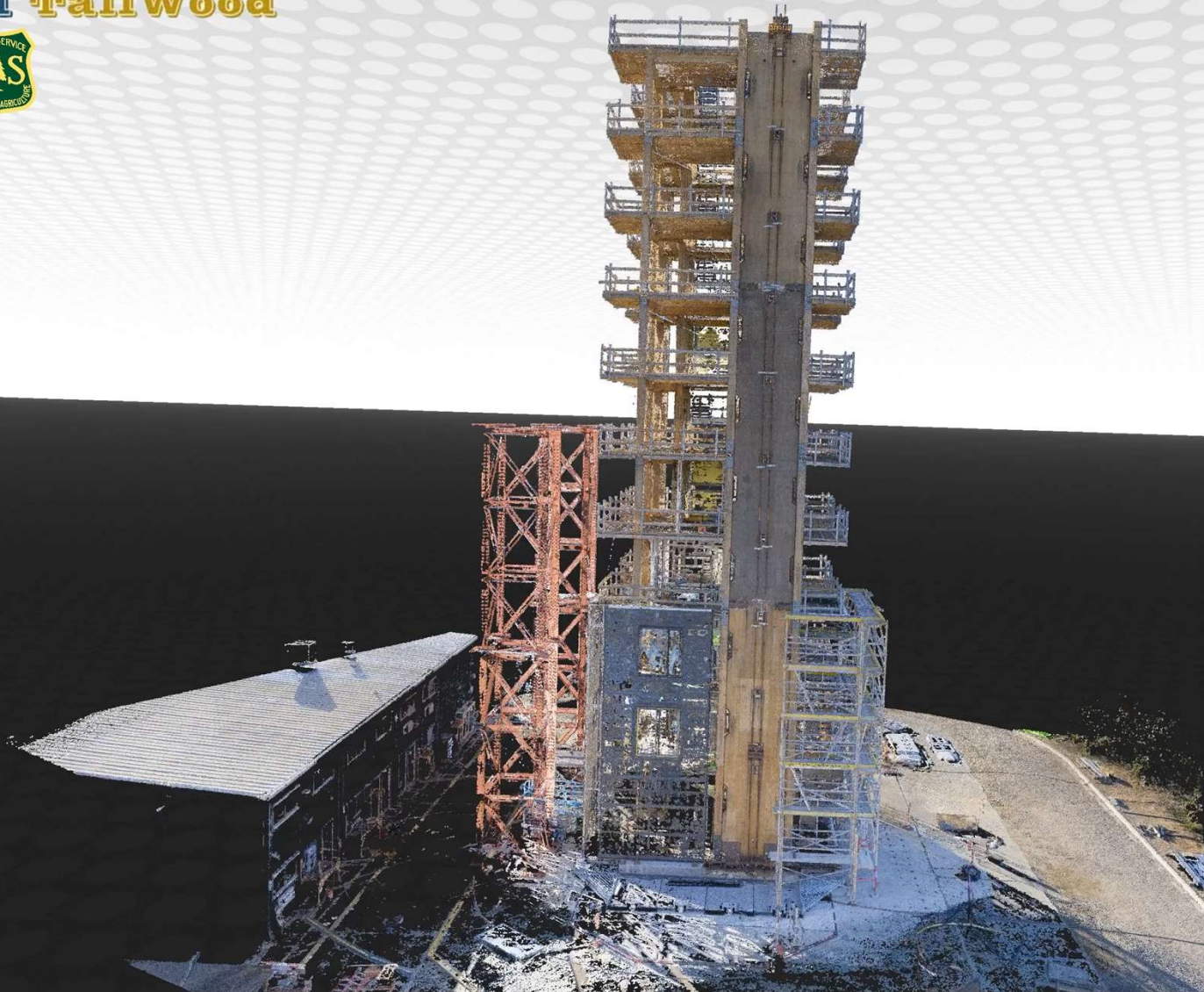
Credit: Prof. Keri Ryan, U Nevada Reno







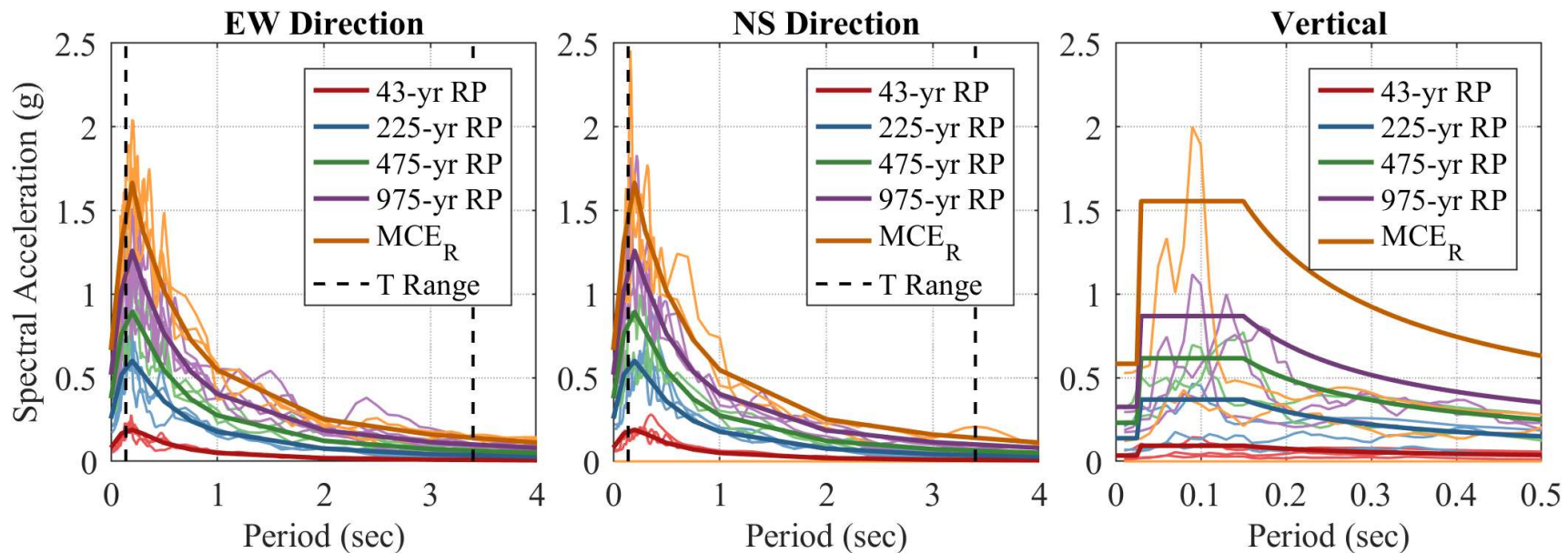
NHERI TallWood



Test Program

Are you going to collapse it?

Wichman (2023). "Seismic Behavior of Tall Rocking Mass Timber Walls" Ph.D. thesis, University of Washington, Seattle, Washington.



Ground motion selected to represent different return periods at the design location (Seattle WA)
Motions also represent different seismic sources (crustal, subduction interface, subduction intraslab)
Motions include uniaxial, biaxial, and 3D

How many earthquakes should we run?

Total 88 Seismic Tests in Phase 1

Test Program (Cont.)

5/1/23

Small shake

225 Yr

DBE

Small & 225 Rerun

5/9/23

Media Day!

DBE

Rerun

975 Yr

MCE

Re-runs

5/22/23

Phase 1 Completed

Hazard Level	Design Set #	Record ID	Earthquake Name	Source
43	3	4213	Niigata, Japan	Crustal
	6	CHBH041103111446	Tohoku	Interface
	7	HKD1310309260450	Tokachi	Interface
	10	subRSN2000890	Ferndale	Intraslab
	11	subRSN2000905	Ferndale	Intraslab
225	3	4213	Niigata, Japan	Crustal
	6	CHBH041103111446	Tohoku	Interface
	7	HKD1310309260450	Tokachi	Interface
	10	subRSN2000890	Ferndale	Intraslab
	11	subRSN2000905	Ferndale	Intraslab
475	2	2951	Chi-Chi	Crustal
	3	3471	Chi-Chi	Crustal
	4	4213	Niigata, Japan	Crustal
	6	CHBH041103111446	Tohoku	Interface
	10	subRSN2000890	Ferndale	Intraslab
975	2	268	Victoria, Mexico	Crustal
	4	964	Northridge-01	Crustal
	5	CHBH041103111446	Tohoku	Interface
	6	HKD1270309260450	Tokachi	Interface
	11	subRSN2000890	Ferndale	Intraslab
MCE _R	3	4228	Niigata, Japan	Crustal
	5	761	Loma Prieta	Crustal
	7	CHBH041103111446	Tohoku	Interface
	8	HKD1270309260450	Tokachi	Interface
	11	subRSN2000890	Ferndale	Intraslab
Media	Media Day*	964	Northridge-01	Crustal

A suite of historical ground motions were selected and scaled to represent different hazard levels. These were also used in the RBSD of the building

An unique opportunity to apply the design suite for actual tests.

Test ID	Hazard Level	Direction	EQ Source	Record Number	Record ID	Earthquake Name	UCSD File	dt	Sig. Horiz.	Sig. Vert.	Total Duration	Target Table Scale Horiz.	Target Table Scale Vert.	X Comp.	Y Comp.	Z Comp.
1	43	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.213	0.000	0.0000	0.0684	0.0000
2	43	Y	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.213	0.000	0.0000	0.0684	0.0000
3	43	XY	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.213	0.000	0.0655	0.0684	0.0000
4	43	X	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	0.319	0.000	0.0525	0.0000	0.0000
5	43	Y	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	0.319	0.000	0.0000	0.0684	0.0000
6	43	XY	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	0.319	0.000	0.0525	0.0684	0.0000
7	43	X	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	0.312	0.000	0.0689	0.0000	0.0000
8	43	Y	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	0.312	0.000	0.0000	0.0689	0.0000
9	43	XY	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	0.312	0.000	0.0655	0.0689	0.0000
10	43	XYZ	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	27.3	80.9	0.213	0.213	0.0655	0.0684	0.0638
11	225	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.2102	0.0000	0.0000
12	225	Y	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.0000	0.2102	0.0000
13	225	XY	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.0655	0.2102	0.0000
14	225	X	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	1.000	0.000	0.1686	0.0000	0.0000
15	225	Y	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	1.000	0.000	0.0000	0.1686	0.0000
16	225	XY	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	1.000	0.000	0.1686	0.1686	0.0000
17	225	X	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.2213	0.0000	0.0000
18	225	Y	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.0000	0.2213	0.0000
19	225	XY	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.2213	0.2213	0.0000
20	475	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	1.000	0.000	0.3150	0.0000	0.0000
21	475	Y	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	1.000	0.000	0.0000	0.3150	0.0000
22	475	XY	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	1.000	0.000	0.3150	0.3150	0.0000
23	475	X	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.4671	0.0000	0.0000
24	475	Y	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.0000	0.4671	0.0000
25	475	XY	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.4671	0.4671	0.0000
26	225	X	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.2102	0.0000	0.0000
27	Media Day*	X	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	0.0	39.8	0.716	0.000	0.9488	0.0000	0.0000
28	Media Day*	Y	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	0.0	39.8	0.716	0.000	0.0000	0.9488	0.0000
29	Media Day*	XY	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	0.0	39.8	0.716	0.000	0.9488	0.9488	0.0000
30	225	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.2102	0.0000	0.0000
31	Media Day*	XYZ	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	20.2	39.8	0.716	0.358	0.9488	0.2516	0.0615
32	Media Day*	X	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	20.2	39.8	0.716	0.358	0.9488	0.2516	0.1230
33	Media Day*	XYZ	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	20.2	39.8	0.716	0.716	0.9488	0.2516	0.1230
34	475	X	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.0000	0.0000	0.0000
35	43	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.213	0.000	0.0655	0.0000	0.0000
36	43	Y	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.213	0.000	0.0000	0.0655	0.0000
37	43	XY	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.213	0.000	0.0655	0.0655	0.0000
38	43	X	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	0.319	0.000	0.0525	0.0000	0.0000
39	43	Y	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	0.319	0.000	0.0000	0.0525	0.0000
40	43	XY	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	0.319	0.000	0.0525	0.0525	0.0000
41	43	X	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	0.312	0.000	0.0689	0.0000	0.0000
42	43	Y	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	0.312	0.000	0.0000	0.0689	0.0000
43	43	XY	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	0.312	0.000	0.0655	0.0689	0.0000
44	43	XYZ	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	27.3	80.9	0.213	0.213	0.0655	0.0684	0.0638
45	225	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.2102	0.0000	0.0000
46	225	Y	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.0000	0.2102	0.0000
47	225	XY	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	0.667	0.000	0.0655	0.2102	0.0000
48	225	X	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	1.000	0.000	0.1686	0.0000	0.0000
49	225	Y	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	1.000	0.000	0.0000	0.1686	0.0000
50	225	XY	Interface	6	CHBH041	Tohoku	225_CHBH	0.01	68.3	0.0	300.0	1.000	0.000	0.1686	0.1686	0.0000
51	Media Day*	X	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	0.0	39.8	0.716	0.000	0.9488	0.0000	0.0000
52	Media Day*	Y	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	0.0	39.8	0.716	0.000	0.0000	0.9488	0.0000
53	Media Day*	XY	Crustal	Media Day	964	Northridge	975_964	0.01	23.0	0.0	39.8	0.716	0.000	0.9488	0.9488	0.0000
54	225	X	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.2213	0.0000	0.0000
55	225	Y	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.0000	0.2213	0.0000
56	225	XY	Crustal	3	4213	Niigata	225_4213	0.01	9.4	0.0	165.9	1.000	0.000	0.2213	0.2213	0.0000
57	225	XYZ	Crustal	3	4213	Niigata	225_4213	0.01	9.4	34.2	165.9	1.000	1.000	0.2213	0.3197	0.0697
58	475	X	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.4671	0.0000	0.0000
59	475	Y	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.0000	0.4671	0.0000
60	475	XY	Crustal	3	3471	Chi-Chi	475_3471	0.005	24.6	0.0	76.0	1.000	0.000	0.4671	0.4671	0.0000
61	475	X	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	1.000	0.000	0.3150	0.0000	0.0000
62	475	Y	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	1.000	0.000	0.0000	0.3150	0.0000
63	475	XY	Intraslab	11	subRSN2	Ferndale	475_subR	0.005	22.0	0.0	80.9	1.000	0.000	0.3150	0.3150	0.0000
64	475	X	Interface	6	CHBH041	Tohoku	MCE_CHBH	0.01	65.1	0.0	300.0	0.529	0.000	0.3960	0.0000	0.0000
65	975	X	Crustal	4	964	Northridge	975_964	0.01	23.0	0.0	39.8	1.000	0.000	0.4889		

MCE: Loma Prieta EQ (0.76 g)



Gravity Column



Corner of the West Rocking wall





NS performance: Effective low damage detail (MCE)

You can see the NS façade returned to original position after shake





We did not break a single window



L6-16N

L6-16E

L6-15E

Initial Observations

- Building periods are relatively long (about 1.8-2 sec)
- After all 88 earthquake loading, No structural damage, No residual drift.
- Further tests for a total of more than 200 earthquakes, rocking wall element and gravity framing essentially undamaged
- Large overall building deformation, rocking mechanism engaged at intensity levels above DBE, reached close to 3% max drift
- Low-damage NS system details are effective, did not break a single window.

First Paper to JSE:

[“Shake Table Test of a Full-Scale Ten-Story Resilient Mass Timber Building”](#)



More testing...

- Payload test by MTU (June 2023) **+19 shakes**
- Phase 2 Japanese test (August 2023) **+51 shakes**
- Deconstruction to 6-story

Prof. Andre Barbosa from OSU (3 phases of testing)

[Converging Design – Home – TallWood Design Institute](http://tallwoodinstitute.org)

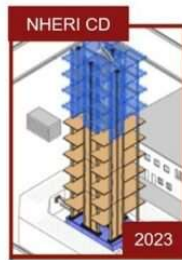
tallwoodinstitute.org **+ 66 shakes (first 2 config)**

So far 88+19+51+66=224 earthquakes



Courtesy of Shiling Pei and Reid Zimmerman

NHERI TallWood
10-story Building Specimen



NHERI Converging Design
6-story Building Specimen



Codification underway...



Previous Wood Innovation Fund

Development of prescriptive design provisions and commentary for post-tensioned mass timber rocking wall lateral systems.

(2018-2023) **Completed**



**CHARLES PANKOW
FOUNDATION**

Building Innovation through Research



FEMA P695 Study on post-tensioned mass timber rocking wall lateral systems
(2024-2026) **on going**



(Mandatory) Requirements for Post-Tensioned Mass Timber Rocking Walls

C.1 General

C.1.1 Scope

These provisions shall be used for the design and construction of all members and connections that are part of a post-tensioned mass timber rocking wall lateral force-resisting system (LFRS) including mass timber panels, mass timber bounding columns (where present), post-tensioning components, and energy dissipation components. The provisions provided herein shall be applied in combination with the requirements of this Standard, the NDS including Appendix E, ASCE 7-16, and the applicable building code.

C.1.2 Notation

A_{vt} = area of vertical lamination of mass timber

$F_{pi,j}$ = initial force in post-tensioning components for wall segment j , lb.

$F_t(A_{parallel})^*$ = adjusted tension design value for mass timber panel composed of cross-laminated timber, lb.

$M_{ed,rm,j}$ = moment capacity of the rocking mechanism contributed by all the energy dissipation components for wall segment j , lb-in.

$M_{n,rm,j}$ = nominal moment capacity of the rocking mechanism for wall segment j , lb-in.

$M_{n,rm,j}^*$ = nominal moment capacity of the rocking mechanism neglecting energy dissipation components for wall segment j , lb-in.

$M_{pr,rm,j}$ = probable moment capacity of the rocking mechanism for wall segment j , lb-in.

Busch et al. (2021) "Prescriptive Seismic Design Procedure for Post-Tensioned Mass Timber Rocking Walls", Journal of Structural Engineering.

Prescriptive Seismic Design Procedure for Post-Tensioned Mass Timber Rocking Walls

A. Busch, S.M.ASCE¹; R. B. Zimmerman, M.ASCE²; S. Pei, M.ASCE³; E. McDonnell, M.ASCE⁴;
P. Line, M.ASCE⁵; and D. Huang, S.M.ASCE⁶

Abstract: In this study, a prescriptive seismic design procedure for post-tensioned mass timber rocking wall lateral force-resisting systems is proposed. Unlike performance-based design approaches that employ nonlinear analysis, this procedure utilizes techniques and analysis procedures that are routinely applied in design industry practice as well as adhering to traditional approaches contained in current US standards. The design procedure targets providing a basis for prescriptive design of mass timber rocking wall lateral force-resisting systems and their future adoption into model codes. For illustration, the design approach is applied to an example building, with the building's performance validated through a nonlinear numerical model simulation. DOI: 10.1061/(ASCE)ST.1943-541X.0003240. © 2021 American Society of

ASCE7 update (2028)

Thank You

Earthquake-proof Tall Buildings



Yes.

Mass Timber + Rocking Wall

At least we know for sure this is one of the ways to do it.



And we are working to codify it in ASCE7 and SPDWS

Twitter: @slpei



NHERI TallWood



University of California at San Diego



Special Thanks to UCSD
Drone Lab for great footage!



Happiness Grows from Trees



Boise Cascade®



The structural system scope of this project is sponsored by NSF Grants No. 1635227, 1634628, 1634204. The nonstructural component scope of this project is sponsored by NSF Grant No. CMMI-1635363 and USFS Grant No. 19-DG-11046000-16. The use and operation of NHERI shake table facility is supported by NSF through Grant No. CMMI-2227407. The test program also received great technical, financial, and material donation support from industry leaders both with the U.S. and internationally; please see the full list of sponsors at <http://nheritallwood.mines.edu/collaboration.html>. The project team is very grateful for this support.